



HAL
open science

Discrete modeling of penetration tests in constant velocity and impact conditions

Quoc Anh Tran, Bastien Chevalier, Pierre Breul

► **To cite this version:**

Quoc Anh Tran, Bastien Chevalier, Pierre Breul. Discrete modeling of penetration tests in constant velocity and impact conditions. *Computers and Geotechnics*, 2016, 71, pp.12-18. 10.1016/j.compgeo.2015.08.010 . hal-01254527

HAL Id: hal-01254527

<https://uca.hal.science/hal-01254527>

Submitted on 12 Jan 2016

HAL is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

Discrete modeling of penetration tests in constant velocity and impact conditions

Quoc Anh Tran, Bastien Chevalier and Pierre Breul

Correspondence to bastien.chevalier@univ-bpclermont.fr

Clermont Université, Université Blaise Pascal, Institut Pascal, BP 10448, F-63000 Clermont-Ferrand, France.

CNRS, UMR 6602, Institut Pascal, F-63171 Aubière, France.

Tel. +33(0)4.73.40.75.23

Fax. +33(0)4.73.40.74.94

Abstract

The paper presents investigations on the penetration tests in granular material. A discrete numerical study is proposed for the modeling of penetration tests in constant velocity conditions and also in impact conditions. The model reproduces qualitatively the mechanical response of samples of granular material, compared to classical experimental results. Penetration tests are conducted at constant velocity and from impact, with similar penetration rates ranging from $25 \text{ mm}\cdot\text{s}^{-1}$ to $5000 \text{ mm}\cdot\text{s}^{-1}$. In constant velocity condition, the value of tip force remains steady as long as the penetration velocity induces a quasi-static regime in the granular material. However, the tip force increases rapidly in the dense flow regime corresponding to higher penetration rate. Impact tip force increases with the impact velocity. Finally, the tip forces obtained from impact penetration tests are smaller compared to the one obtained in constant velocity conditions in both quasi-static and dense flow regimes.

Keywords: DEM, Penetration test, Tip force, Penetration rate

Computers and Geotechnics 71 (2016) 12–18

<http://dx.doi.org/10.1016/j.compgeo.2015.08.010>

Article history:

Received 13 February 2015

Received in revised form 18 August 2015

Accepted 31 August 2015

31 1. Introduction

32 In the field of in situ mechanical characterization of soils, penetration tests are commonly
 33 used. The tip resistances, deduced from pile driving theory, can be measured either in
 34 dynamic (q_d) (Fig.1) or in static conditions (q_c).

35 Recently, the measurement technique in impact conditions was improved. It is now possible
 36 to record the real-time response of the soil during one impact in terms of tip force and
 37 penetration distance [1,2] (Fig.2). Mechanical properties other than the classical tip resistance
 38 might be extracted from this new kind of experimental measurements. Recent studies from [3]
 39 and [4] showed the interest in penetration tests for the characterization of coarse material.

40 Penetration tests generate large deformations and a highly non-homogeneous solicitation,
 41 Discrete Element Method (DEM) is then a particularly relevant numerical method to model
 42 this test. Many authors proposed numerical models for reproducing penetration tests in static
 43 conditions i.e. in constant velocity conditions in 2D [5,6,7,8,9,10] and in 3D [1,4,11,12].
 44 However, [1,13,14] showed that tip resistance depends on the loading type used in the
 45 penetration process. Very few researches focus to the modeling of penetration tests in impact
 46 conditions.

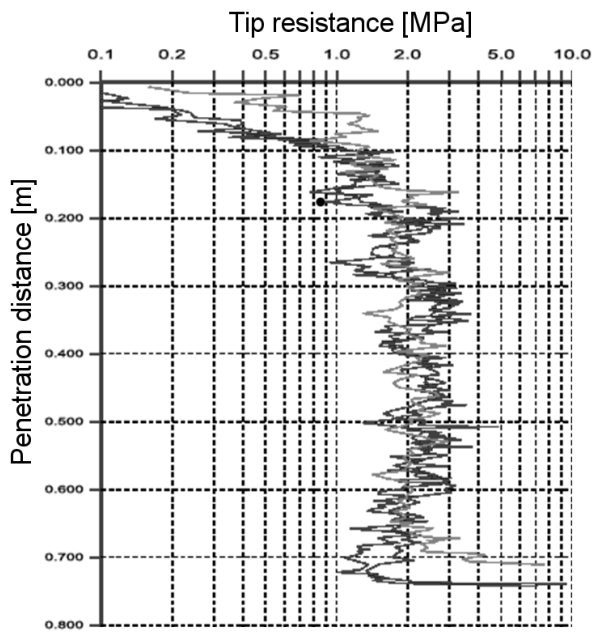


Figure 1. Example of an experimental result of a impact penetration test.

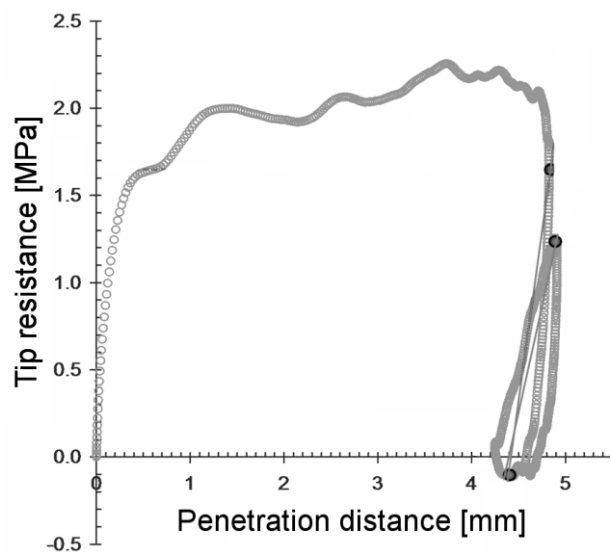
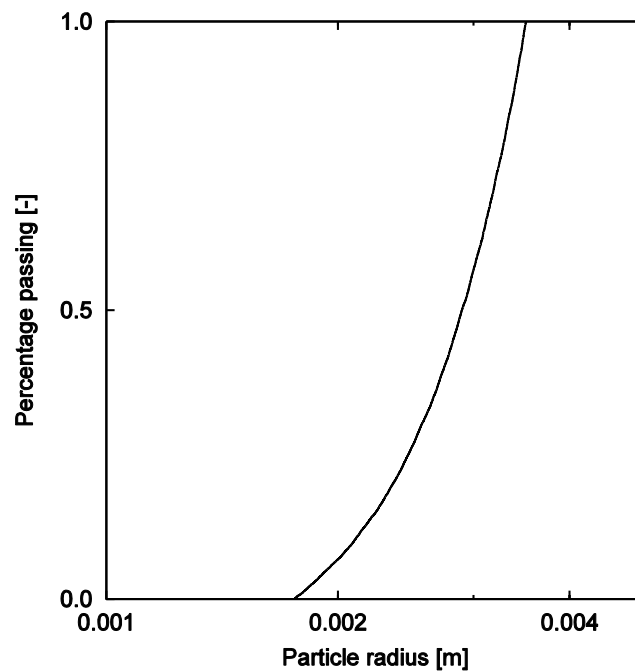


Figure 2. Example of experimental load-penetration curve obtained in a impact penetration test for one impact [2].

47 In this paper, we propose a numerical model of penetration tests using DEM for reproducing
48 tests in both constant velocity and impact conditions in coarse materials. The penetration
49 device modeled here is a light penetrometer [3,4]. Macroscopic results are discussed in this
50 paper. After the description of the numerical model, we present the effect of penetration rate
51 on the tip force obtained from both constant velocity and impact penetration tests will.
52 Finally, a comparison of the tip force obtained with both loading types is proposed and
53 discussed.

54 2. Numerical Model

55 Discrete Element Method in two dimensions was used with Itasca's software PFC^{2D} [15].
56 Table 1 summarize the parameter of the model. Granular material samples of 10 000
57 cylindrical particles were generated and tested in a rectangular box (Table 1). A diameter ratio
58 of 2 was chosen between largest and smallest particles. The average particle diameter of the
59 material D_p is equal to 5.4 mm (Fig.3).



60
61

Figure 3. Particle size distribution of the granular material.

62 The sample preparation broke down into 3 steps. First, a frictionless particle radius expansion
63 method without gravity was used in order to reach a minimum value of sample porosity of
64 $n = 0.15$. Secondly, the final value of friction coefficient of $\mu_{particle} = 1.00$ was applied as well
65 as the gravity. We conducted simulations with different values of particle friction and found
66 no influence of particle friction on the results for values of $\mu_{particle} \geq 0.50$. So the value of
67 $\mu_{particle} = 1.00$ was chosen. The sample was then stabilized until equilibrium state was reached.

68 At the end of this step, the internal stress state at center of the sample was calculated. The
 69 ratio between horizontal and vertical stresses was found equal to 0.5, which is close to
 70 classical “at rest” earth pressure ratio K_0 . This ratio was also calculated from the stresses
 71 measured on sample boundaries. Finally, the sample was confined vertically on its top
 72 surface.

73 Usually in homogeneous soils, tip resistance first increases with depth until a critical depth is
 74 reached and then tip resistance becomes steady (Fig.1). The confining stress, equal to 40 kPa
 75 simulates an overlaying layer of material; it prevented the effects of free surface to be
 76 observed [14]. A linear contact model was used and the contact stiffness was chosen in order
 77 to assess the assumption of rigid particles during penetration tests [16,17]. A Coulomb
 78 friction criterion of coefficient $\mu_{particle} = 1.00$ was used to limit the value of tangential force
 79 relatively to normal force. No viscous damping was considered in the contact model and no
 80 local damping was used in the model [18]. Thus, energy is only dissipated by friction during
 81 the penetration tests.

<i>Parameter</i>	<i>Symbol</i>	<i>Value</i>	<i>Unit</i>
Width box	L	0.6	m
Height box	H	0.45	m
Particle number	N_P	10 000	–
Average particle diameter	D_p	5.4	m
Particle density	ρ	2 700	kg.m ⁻³
Normal contact stiffness	k_n	1.25 x 10 ⁸	N/m
Tangential contact stiffness	k_s	9.375 x 10 ⁷	N/m
Particle friction coefficient	$\mu_{particle}$	1.00	-
Rod friction coefficient	μ_{rod}	0.00	-
Tip friction coefficient	μ_{tip}	0.30	-

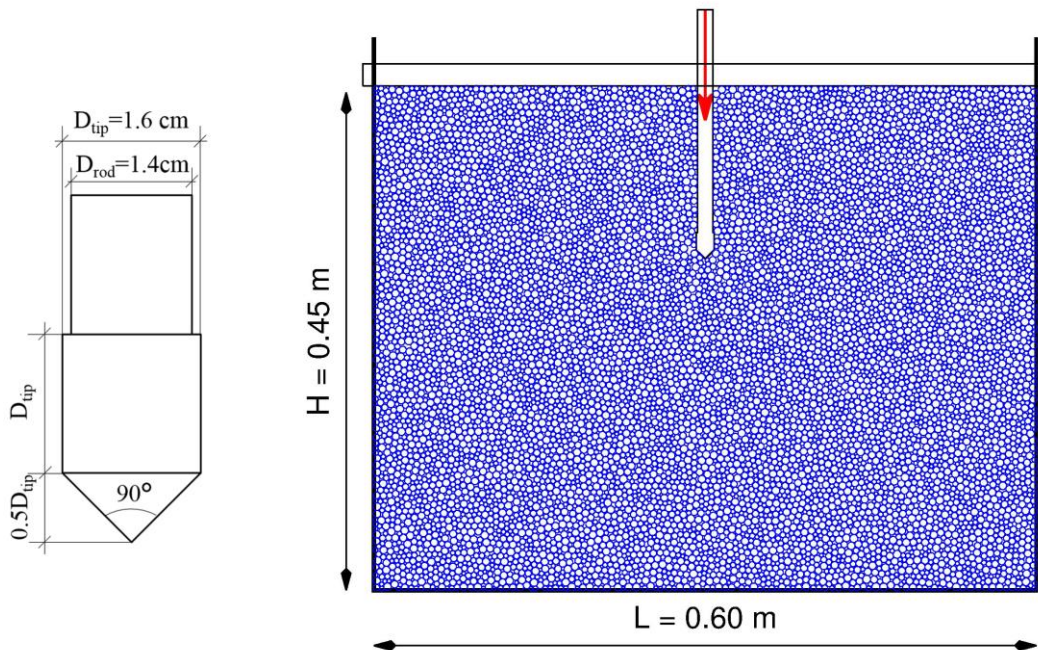
82 Table 1. A summary table with all DEM parameters used in penetration tests.

83 Penetration tests were conducted on three different samples generated with the same
 84 conditions of density and particle grading but different initial particle arrangement. The
 85 penetration was performed with a frictionless rod of width 14 mm linked to a tip of 16 mm
 86 width at its bottom edge and presenting a friction coefficient μ_{tip} of 0.3 [2,3,4] (Fig.4). In
 87 constant velocity conditions, called hereafter constant velocity conditions test, the rod is
 88 driven in the sample with a constant rod velocity up to 0.30 m of depth. The vertical

89 component of the force applied by the granular material on the tip is called tip force F_c for
90 penetration test conducted in constant velocity condition.

91 For tests conducted in impact conditions, the rod is first driven with constant velocity until a
92 depth of 0.15 m is reached. The rod is then released and stabilized under its own weight.
93 Then, series of five successive impacts are produced in each sample with an additional
94 cylinder on the top of the rod (Fig.4). The mass of the impacting cylinder is equal to the rod
95 mass. The vertical component of the force applied by the granular material on the tip is called
96 tip force F_d in impact condition tests. Equilibrium state is reached after each blow and before
97 applying the next blow.

98 The equilibrium state used in the simulations is a classical equilibrium state condition. Once
99 one of the two ratio values defined hereafter decreases below a given value, the system is
100 considered in mechanical equilibrium. The first ratio is given by the ratio of average
101 unbalanced force magnitude of particles to average magnitude of normal contact force. The
102 second ratio is given by the ratio of the magnitude of the greatest unbalanced force on
103 particles to the magnitude of the greatest normal contact force.



104
105

Figure 4. Tip details and sample of granular material tested.

106 Figure 5 shows the tip force F_c versus the depth in a given sample of 0.60 m width for depth
107 between 0.15 m and 0.30 m, obtained with a rod velocity of 25 mm.s^{-1} . Despite some
108 oscillations, due to coarse nature of the material, it is found that F_c is relatively steady in

109 average as the depth increases and is keeping with an experimental constant velocity
110 penetration test. The upper confining stress cancelled the effect of the free surface.

111 In order to highlight the effect of sample width on the test results, constant velocity
112 penetration tests were conducted in boxes of different width ranging from 0.15 m to 0.90 m.
113 The penetration rate used is equal to $1250 \text{ mm}\cdot\text{s}^{-1}$, which represents an average value of
114 penetration rates used in this study (constant velocity and impact conditions). Figure 6 shows
115 the probability distribution of F_c obtained for samples width varying between 0.15 m and
116 0.90 m. As the box width increases, we observe that the probability distribution of the values
117 of F_c becomes stable when the width is greater than 0.60 m.

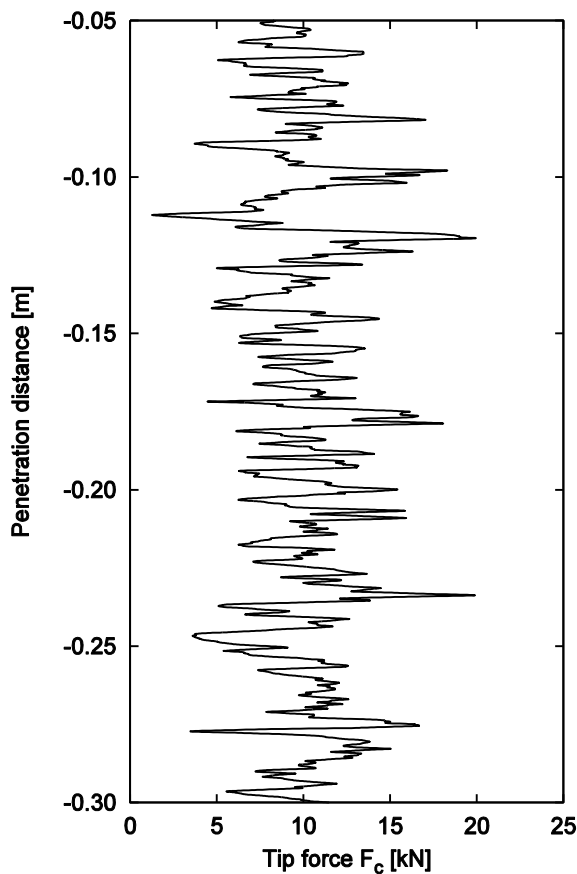


Figure 5. Tip force F_c versus penetration distance obtained at $25 \text{ mm}\cdot\text{s}^{-1}$ of rod velocity in the sample of 0.6 m width.

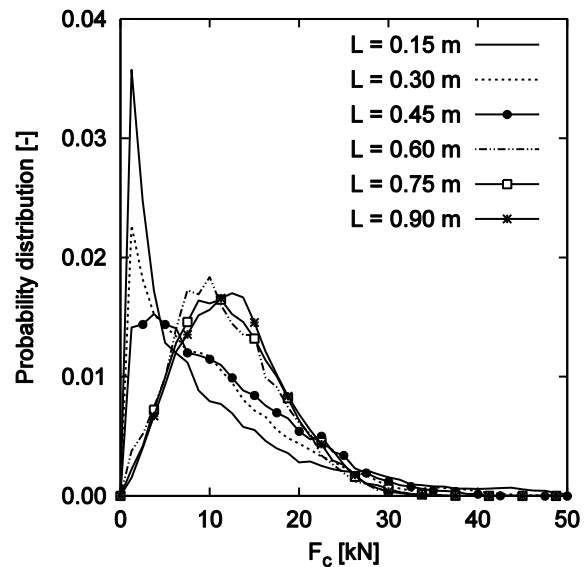


Figure 6. Probability distribution of tip force F_c between 0.05 and 0.30 m of penetration distance at $1250 \text{ mm}\cdot\text{s}^{-1}$ of rod velocity for different samples width L .

118 3. Effect of penetration rate on the tip force in constant
 119 velocity penetration test

120 In this section, we focus on the influence of the driving velocity on the tip force F_c for
 121 constant velocity penetration tests. The penetration rates range from a low value of, 25 mm.s^{-1}
 122 corresponding to penetration rate prescribed in the standards for constant velocity penetration
 123 test to a fast penetration rate corresponding to the order of magnitude of impact velocity used
 124 in impact conditions 5000 mm.s^{-1} as described in [1,2].

125 Figure 7 shows the probability distributions of all values of tip force F_c measured between
 126 0.05 m and 0.30 m of penetration depth obtained for three samples with different penetration
 127 rates [19]. Probability distributions of tip force F_c complies with the normal law when
 128 penetration rate is lower than 1250 mm.s^{-1} . The dispersion of F_c increases when rod velocity
 129 is higher than 1250 mm.s^{-1} .

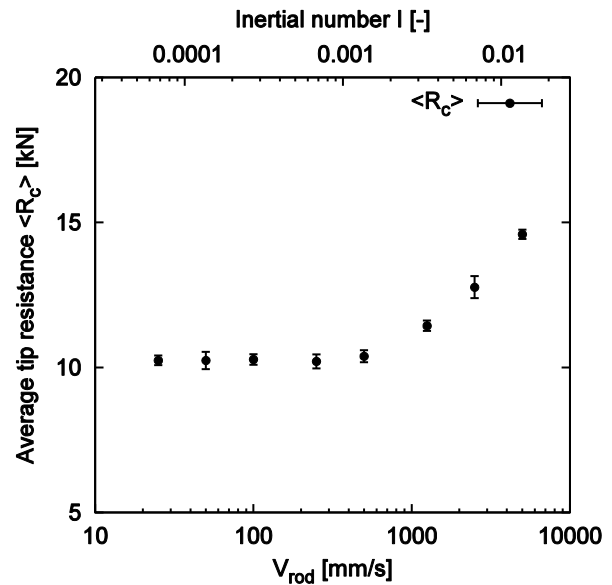
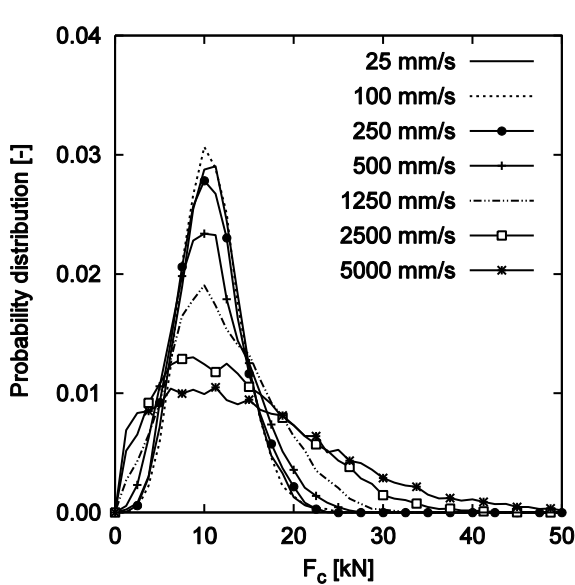


Figure 7. Probability distribution of tip force F_c between 0.05 and 0.30 m of penetration distance for three samples with different rod velocities [19].

Figure 8. Average tip resistance $\langle R_c \rangle$ versus rod velocity (height of vertical bars represent twice the standard deviation of R_c).

130 The non-dimensional inertial number I can be used to quantify dynamic effects in both
 131 experimental tests and numerical modeling [17]. Inertial number is given by

$$I = \dot{\gamma} \sqrt{\frac{m}{P}} \quad (1)$$

132 with $\dot{\gamma}$ the shearing rate of the particle assembly during penetration testing, m the average
 133 particle mass and P the confinement stress. It can be used to differentiate the regimes of

134 solicitation: from quasi-static state with $I < 10^{-3}$ to inertial state with $I > 10^{-3}$ [17]. It is
135 difficult to determine the shearing rate for penetration tests since the deformation applied to
136 the material is highly non-homogeneous. In order to get an order of magnitude of the inertial
137 number, the deformation rate is calculated by the formula being proposed based on V_{rod} the
138 rod velocity; H the sample height:

$$\dot{\gamma} = \frac{V_{rod}}{H} \quad (2)$$

139 The inertial number I defined by Eq.1 & 2 increases from 6.80×10^{-5} to 1.36×10^{-2} according to
140 the penetration rate (from 25 mm.s^{-1} up to 5000 mm.s^{-1}).

141 The tip resistance R_c is defined here as the average of F_c obtained between 0.05 and 0.30 m of
142 penetration distance in a given sample. The average tip resistance $\langle R_c \rangle$ obtained on three
143 different samples is calculated.

144 It can be observed on Fig.8 that $\langle R_c \rangle$ remains constant when the rod velocity is lower than
145 1250 mm.s^{-1} . Then, $\langle R_c \rangle$ increases rapidly for penetration rate upper than
146 1250 mm.s^{-1} corresponding to an inertial number I (in the order of 3.40×10^{-3}). It can also be
147 noticed on Fig.7 that the dispersion of tip force F_c also increases with rod velocity.

148 The same trend was described in [4]. In this paper, tip resistance q_c is steady for low value of
149 penetration rates and then increases as penetration rate increases. In both studies, the change
150 of regime occurs for different values of the rod velocity, because this value probably depends
151 on particle size distribution, tip size, confining stress P (as show in Eq.1&2) and possibly
152 additional parameters.

153 4. Effect of penetration rate on the tip force in impact 154 penetration test

155 For impact penetration tests, impacts are generated on the top of the driving rod and the tip
156 force F_d is measured as well as the penetration distance.

157 The effect of impact energy is significant in impact penetration tests. The impact test were
158 compared in terms of maximal rod velocity and not in terms of impact velocity. In order to
159 show that rod maximal velocity is dependent on impact energy, impact tests were conducted
160 with same impact energy but with changing impact mass and impact velocities The ratio
161 between impact mass and rod mass (ξ) for successively taken equal to 0.5, 1.0 and 2.0. Figure

162 9 presents the three curves of versus penetration distance obtained. First, the magnitude of F_d
 163 is similar for 3 cases. Furthermore, the same maximum rod velocity $V_{rodmax} \cong 1210 \text{ mm.s}^{-1}$ is
 164 obtained in the different cases corresponding to different ratios ξ (Fig.10).

165 Secondly, the response obtained with the model is similar to the one classically obtained
 166 experimentally (Figure 2), it breaks down into three phases (Fig.9,10):

- 167 • a quick loading phase corresponding to the initial increase of the rod velocity. In this
 168 phase, whatever is the blow, the signal shape is similar. The duration of this phase is
 169 the same as the duration of the impact ($t_{impact} \approx 2.2 \text{ ms}$). The first point (1)
 170 corresponds to the time when the rod velocity reaches its maximum velocity.
- 171 • a plastic phase corresponding to the penetration process of the rod in the soil. In this
 172 phase, the signal shows oscillations depending on the arrangement of the granular
 173 material. The second point (2) corresponds to a moment in this phase when the rod
 174 velocity decreases. The point (3) corresponds to the moment when the penetration
 175 distance is maximal: the rod velocity is equal to zero.
- 176 • a phase of unloading–loading cycles corresponding to the stabilization of the rod. The
 177 fourth point (4) shows the moment when the rod velocity is zero for second time.

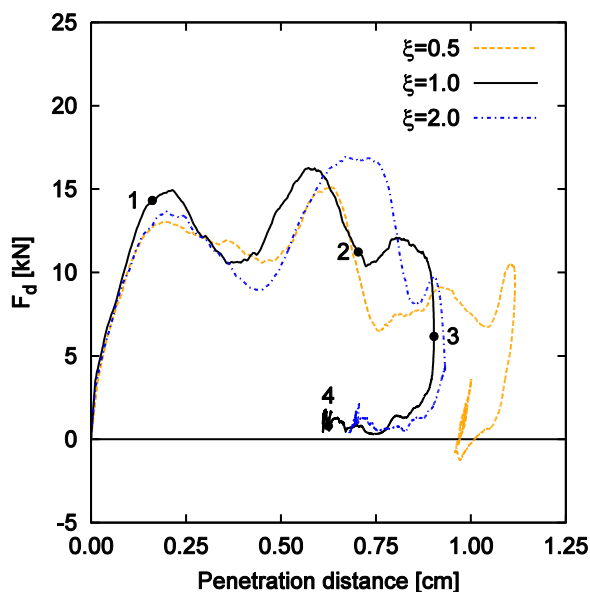


Figure 9. Examples of load–penetration curves obtained for 3 tests performed with the same impact energy.

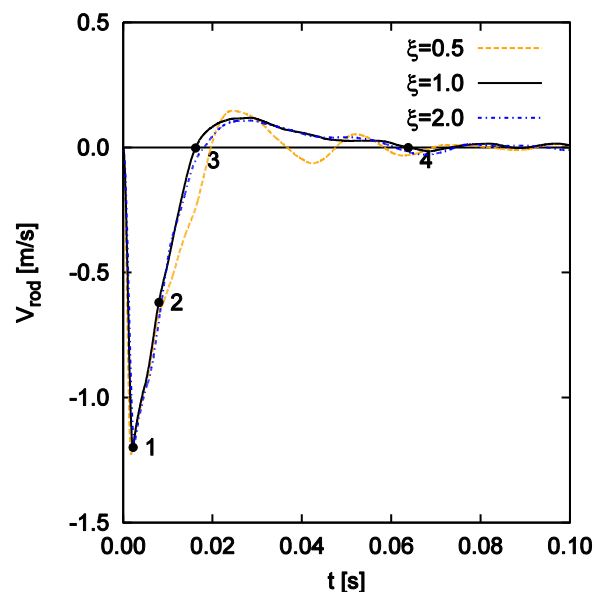
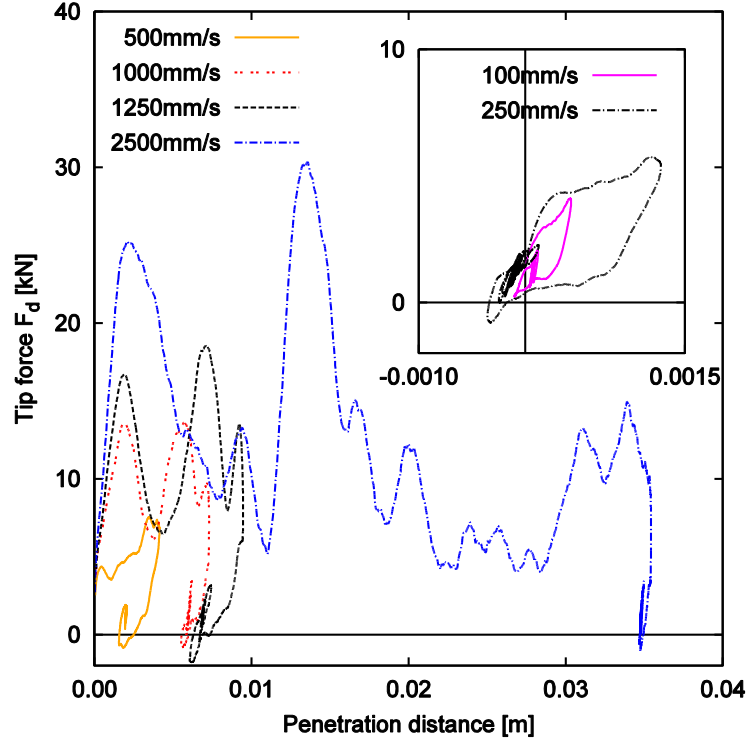


Figure 10. Rod velocity versus time during an impact penetration test for for 3 tests performed with the same impact energy.

178 Figure 11 shows the load–penetration curve for different impact velocities ($\xi = 1.0$). For an
 179 impact velocity of 250 mm/s or smaller, the energy injected is not large enough to drive the
 180 rod in the medium: at the end of the impact test, the tip comes back to its initial position; the
 181 tip force first increases and then rapidly decreases; the plastic phase of load–penetration curve

182 (Fig.9) is not observed. For impact velocity of $500 \text{ mm}\cdot\text{s}^{-1}$ or greater, the tip does not come
 183 back completely to its initial position. Figure 11 shows that the minimal velocity required to
 184 penetrate the granular material is a value between 250 and $500 \text{ mm}\cdot\text{s}^{-1}$. When the impact
 185 velocity is greater than $500 \text{ mm}\cdot\text{s}^{-1}$, the plateau of the load–penetration curve corresponding to
 186 the plastic phase is observed.



187

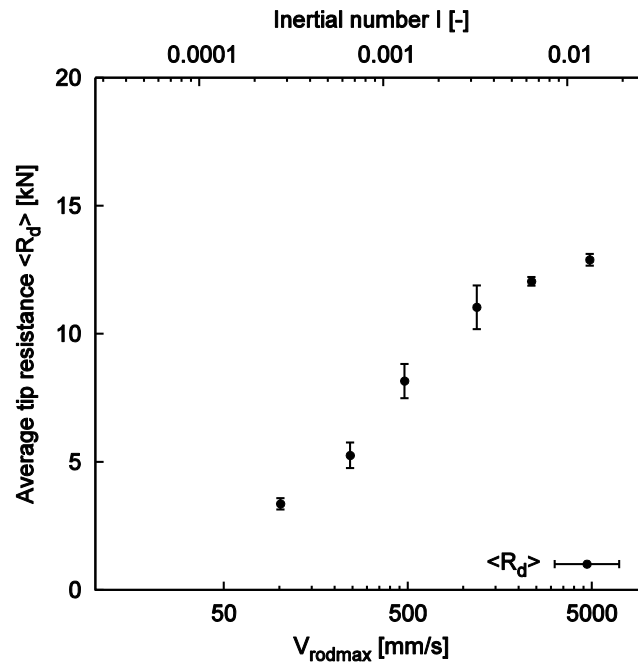
188 Figure 11. Load versus penetration distance for different impact velocities for impact penetration test.

189 Although, there is a difference between the maximal penetration distance s_{max} and the final
 190 residual penetration distance s_{res} due to the rebound of the rod at the end of the test, the work
 191 of the tip force between these two positions is negligible. Consequently, the impact tip
 192 resistance R_d of each sample was calculated as the average tip force F_d for penetration
 193 distance between 0 and maximal value s_{max} :

$$R_d = \frac{1}{5} \sum_{i=1}^5 \left\{ \frac{1}{s_{max}} \int_{t=0}^{t_{s_{max}}} F_d(t) ds(t) \right\}_i \quad (3)$$

194 with t the time and $t_{s_{max}}$ the time when penetration distance is maximal and equal to s_{max} .

195 $\langle R_d \rangle$ is the average value of impact tip resistances obtained on 3 samples. Figure 12 shows
 196 the curve of $\langle R_d \rangle$ versus maximal rod velocity V_{rodmax} for different impact energy. We find
 197 that $\langle R_d \rangle$ increases when the rod velocity increases.

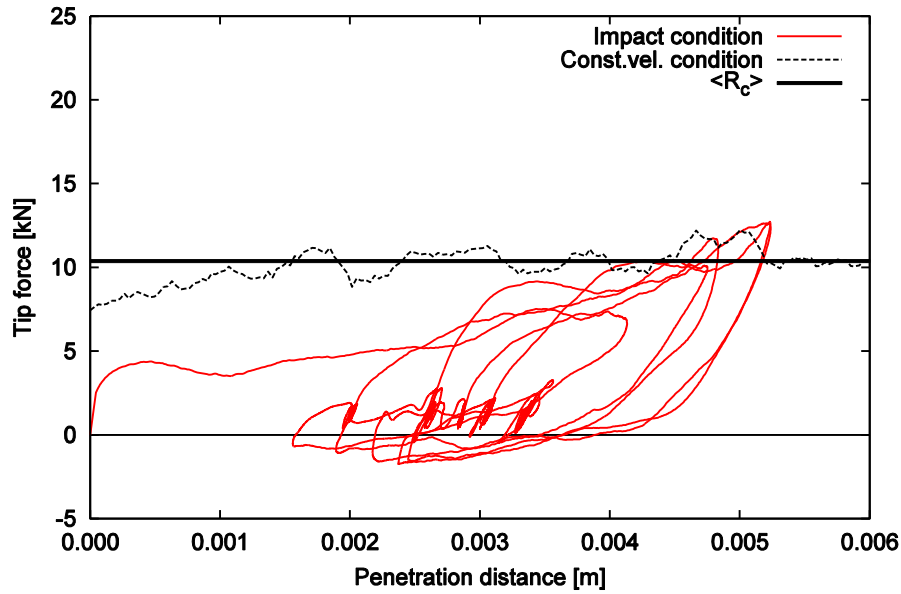


198

199 Figure 12. Average tip resistance $\langle R_d \rangle$ versus maximum rod velocities. Upper x-axis shows the corresponding
 200 values of inertial number (height of vertical bars represent twice the standard deviation of R_d).

201 5. Tip force comparison for constant velocity and impact 202 conditions with different rod velocities

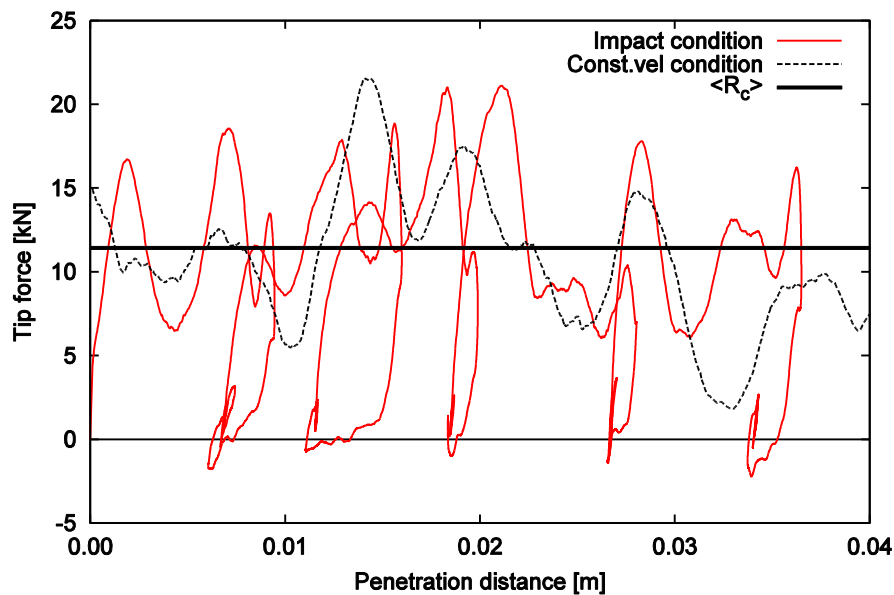
203 Figure 13 presents the tip force versus penetration distance in both constant velocity and
 204 impact conditions for a rod velocity of $500 \text{ mm}\cdot\text{s}^{-1}$, corresponding to the quasi-static regime
 205 of solicitation. We found that the amplitude of tip force F_d is weaker than the average tip
 206 force $\langle R_c \rangle$. This observation is correlated to the fact that in impact conditions, the impact
 207 energy is not sufficient for driving the rod through the granular material. At the end of the
 208 phase 1, F_d reaches the value of F_c but then F_d immediately decreases.



209

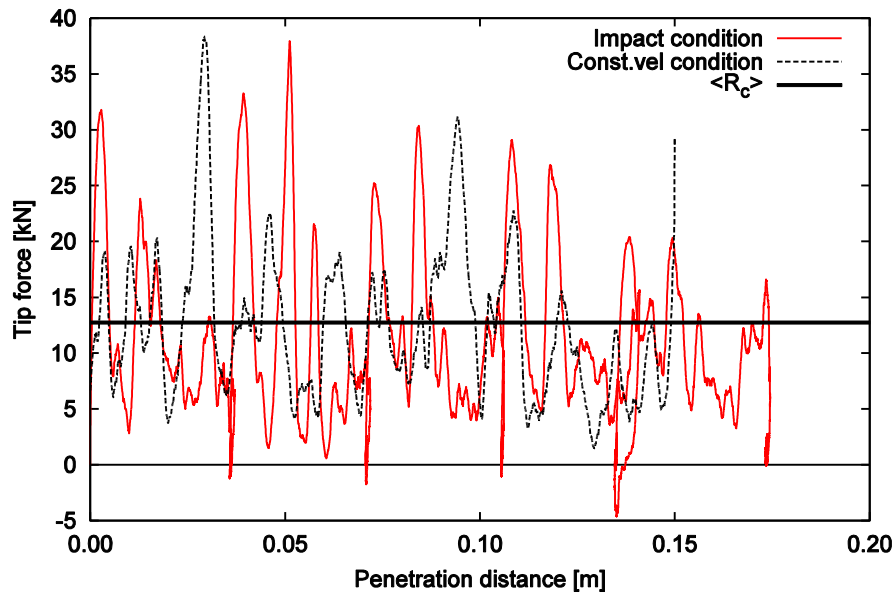
210 Figure 13. Tip force versus penetration distance for constant velocity penetration test with $V_{rod} = 500 \text{ mm.s}^{-1}$ and
 211 for 5 blows of impact penetration tests performed with $V_I = 500 \text{ mm.s}^{-1}$.

212 Figures 14 and 15 present the tip force versus penetration distance in both constant velocity
 213 and impact conditions at 1250 mm.s^{-1} and 2500 mm.s^{-1} of rod velocity range. For this
 214 penetration rate, the particle behavior is in the dense flow regime. In contrast to 500 mm.s^{-1} of
 215 rod velocity range, we get to generate sufficient energy from the impact to activate the plastic
 216 phase. We found that the tip force amplitude is similar in both constant velocity and impact
 217 conditions. In addition, the tip force oscillations become more important when the penetration
 218 rate increases (Fig.14,15).



219

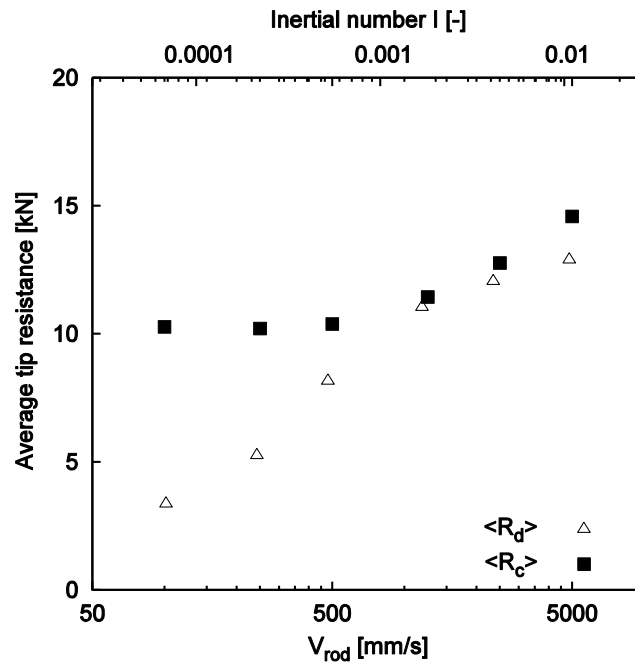
220 Figure 14. Tip force versus penetration distance for constant velocity penetration test with $V_{rod} = 1250 \text{ mm.s}^{-1}$
 221 and for 5 blows of impact penetration tests performed with $V_I = 1250 \text{ mm.s}^{-1}$.



222

223 Figure 15. Tip force versus penetration distance for constant velocity penetration test with $V_{rod} = 2500 \text{ mm.s}^{-1}$
 224 and for 5 blows of impact penetration tests performed with $V_I = 2500 \text{ mm.s}^{-1}$.

225 Figure 16 presents the comparison between the average tip forces obtained in constant
 226 velocity and impact conditions at different penetration rates. In fact, the average tip force in a
 227 homogeneous medium is stable in the zone where the surface effect is prevented by the
 228 vertical confining stress used on top wall. Thus, the average tip force do no depend on the
 229 penetration distance for any penetration condition. We note that $\langle R_d \rangle$ is presented in impact
 230 condition as function of maximal rod velocity V_{rodmax} . In quasi-static regime and for similar
 231 rod velocity, we found that the $\langle R_d \rangle$ is smaller than the one obtained in constant velocity
 232 penetration test. In dense flow regime ($V_{rod} \geq 1250 \text{ mm.s}^{-1}$), $\langle R_d \rangle$ becomes close to $\langle R_c \rangle$. For
 233 high impact energy, the rod velocity in impact condition increases only during the impact.
 234 After that, the rod velocity decreases due to the reaction of the particles below the tip. Thus,
 235 the $\langle R_c \rangle$ can be always greater than $\langle R_d \rangle$ for all rod velocities in dense flow regime.



236

237

Figure 16. Average tip resistances $\langle R_c \rangle$ and $\langle R_d \rangle$ versus rod velocity.

238

239

240

241

242

243

244

245

246

In 3D conditions, we can assume that, as in 2D, an impact energy which is too low can be insufficient to penetrate the material and then to measure a representative tip resistance. On the opposite, for penetration rates high enough, a tip resistance can be measured in impact condition. The increase of tip resistance with the rod velocity was observed in 3D conditions in [4]. In addition, in experimental tests, it is commonly observed that static penetration resistance, measured with low penetration velocity, is lower than dynamic tip resistance, which is measured with relatively high penetration rates. The same trend is observed here on Fig.16: tip resistance in impact condition, for higher rod velocity is greater than tip resistance in constant velocity condition obtained with lower velocity.

247

6. Conclusion

248

249

250

251

A 2-dimensional discrete numerical model was proposed to model penetration tests in granular materials. Two types of tests were performed: constant velocity conditions tests and impact conditions tests. The responses obtained in terms of tip forces versus penetration depth is similar to classical experimental results.

252

253

254

255

256

Penetration test in soils actually is a three-dimensional problem but was simulated here in plane strain or two dimensions in this study. It is true that an assembly of disks cannot capture exactly the behavior of a real granular material. However, the study presented here focuses only on the mechanisms involved in two different types of penetration tests and on the effect of driving velocity. The study presented here has no intention to link directly and

257 quantitatively the results obtained in 2D with 3D modelling or field penetration tests. Yet, the
258 basic laws governing the behavior of a mechanical system such as assemblies of disks or
259 spheres are supposed to be shared between those different kinds of systems. Indeed, number
260 of studies proved 2D DEM to be efficient in describing soil behavior [10]. Also, the basic
261 trends observed here are in agreement with other papers focused on 3D simulations.

262 The effect of penetration rate on constant velocity and impact penetration tests where
263 investigated. The particle behavior changes from quasi-static regime to dense flow regime
264 when rod velocity range varies from 25 mm.s^{-1} to 5000 mm.s^{-1} with a transition value around
265 1250 mm.s^{-1} .

266 In constant velocity condition, the tip force is stable when the rod velocity is lower than
267 1250 mm.s^{-1} . However, the average tip resistance and the dispersion of tip force increase
268 rapidly when the particle behavior in dense flow regime for a tip velocity greater than
269 1250 mm.s^{-1} .

270 In impact condition, the load-penetration curves consists in 3 different phrases. The variation
271 of tip force increases in terms of amplitude when the impact velocity increases. In addition,
272 the energy injected is not large enough to drive the rod in the medium in impact condition
273 when the impact velocity is lower than 500 mm.s^{-1} .

274 Finally, the tip forces obtained from impact and constant velocity penetration tests were
275 compared. In quasi-static regime corresponding to impact velocities less than 500 mm.s^{-1} , the
276 impact energy is not sufficient for driving the rod through the granular material. For greater
277 impact energy, the amplitude of tip force is closer to but lower than average tip resistance
278 $\langle R_c \rangle$ obtained in constant velocity test with the same rod velocity. When comparing constant
279 velocity and impact tests, the rod velocity in impact test is the same as in the constant velocity
280 test only at the beginning of the penetration process; as the tip penetrates the material, its
281 velocity progressively decreases and the resulting tip force is lower.

282 In future tests, it would be interesting to quantify the influence on the results of the contact
283 model and also consider the effect of particle crushing in order to refine the analysis of the
284 results.

285 Reference

286 [1] Benz Navarrete, M. (2009). *Mesures dynamiques lors du battage du pénétromètre*
287 *Panda 2*. PhD thesis, Clermont-Ferrand: Université Blaise Pascal.

- 288 [2] Benz, M.A., Escobar, E., Gourvès, R., Haddani, Y., Breul, P. & Bacconnet, C. (2013).
289 Dynamic measurements of the penetration test - Determination of the tip's dynamic load-
290 penetration curve. *Proc. of the 18th Int. Conf. on the Soil Mech. and Geotech. Eng., Paris*,
291 499 – 502.
- 292 [3] Breul, P., Benz, M., Gourvès, R., & Saussine, G. (2009, June). Penetration Test
293 Modelling in a Coarse Granular Medium. In *Powders and Grains 2009: Proceedings of the*
294 *6th International Conference on Micromechanics of Granular Media* (Vol. 1145, No. 1, pp.
295 173 - 176). AIP Publishing.
- 296 [4] Quezada, J. C., Breul, P., Saussine, G., & Radjai, F. (2014). Penetration test in coarse
297 granular material using Contact Dynamics Method. *Computers and Geotechnics*, 55, 248–
298 253.
- 299 [5] Huang, A. B. & Ma, M. Y. (1994). An analytical study of cone penetration test in
300 granular material. *Canadian Geotechnical Journal* 31, No.1, 91 – 103.
- 301 [6] Huang, A. B. & Hsu, H. H. (2004). Advanced calibration chambers for cone
302 penetration testing in cohesionless soils. *ISC-2 Geotech. and Geophys. Site Characterization*,
303 *Porto*, 147 – 166.
- 304 [7] Calvetti, F. & Nova, R. (2005). Micro-macro relationships from DEM simulated
305 element and in-situ tests. *Proc. 5th Int. Conf. Micromech. Granular Media: Powders and*
306 *Grains 2005, Stuttgart*, 245 – 250.
- 307 [8] Jiang, M. J., Yu, H.-S. & Harris, D. (2006). Discrete element modeling of deep
308 penetration in granular soils. *International Journal for Numerical and Analytical Methods in*
309 *Geomechanics* 30, No. 4, 335 – 361.
- 310 [9] Jiang, M. J., Harris, D., & Zhu, H. (2007). Future continuum models for granular
311 materials in penetration analyses. *Granular Matter*, 9(1), 97–108.
- 312 [10] Jiang, M., Dai, Y., Cui, L., Shen, Z., & Wang, X. (2014). Investigating mechanism of
313 inclined CPT in granular ground using DEM. *Granular Matter*, 16(5), 785-796.
- 314 [11] Arroyo, M., Butlanska, J., Gens, A., Calvetti, F. & Jamiolkowski, M. (2011). Cone
315 penetration tests in a virtual calibration chamber. *Géotechnique* 61, No. 6, 525 – 531.

- 316 [12] McDowell, G. R., Falagush, O. & Yu, H. S. (2012). A particle refinement method for
317 simulating DEM of cone penetration testing in granular materials. *Géotechnique Letters* 2,
318 141 – 147.
- 319 [13] Robertson, P. K., & Campanella, R. G. (1983). Interpretation of cone penetration
320 tests. Part I: Sand. *Canadian Geotechnical Journal*, 20(4), 718-733.
- 321 [14] Chaigneau, L. (2001). *Caractérisation des milieux granulaires de surface à l'aide d'un*
322 *pénétromètre*. PhD thesis. Clermont-Ferrand: Université Blaise Pascal.
- 323 [15] Cundall P. A. & Strack O. D. L. (1979). A discrete numerical model for granular
324 assemblies. *Géotechnique* 29, No. 1, 47 – 65.
- 325 [16] Combe G. (2002) Mécanique des matériaux granulaires et origines microscopiques de
326 la déformation. *Etudes et Recherches du Laboratoire Central des Ponts et Chaussées*, SI8.
- 327 [17] Roux, J.-N., & Chevoir, F. (2005). Discrete numerical simulation and the mechanical
328 behavior of granular materials. *Bulletin Des Laboratoires Des Ponts et Chaussées*, (254),
329 109 – 138.
- 330 [18] Cundall P.A., Drescher A. & Strack O.D.L. (1982). Numerical experiments on
331 granular assemblies: measurements and observations. *IUTAM Conf. on Deformation and*
332 *Failure of Granular Materials, Delft*, 355 - 370.
- 333 [19] Tran, Q. A., Chevalier, B., & Breul, P. (2015). A numerical study of the penetration
334 test at constant rod velocity. *Computer Methods and Recent Advances in Geomechanics –*
335 *Oka, Murakami, Uzuoka & Kimoto (Eds.)*, p. 193-198.